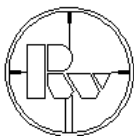


RESIDENTIAL SUBDIVISION
NARRAWALLEE
DRAINAGE REPORT – MAJOR FLOWS

Issue No.0
October 2010



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1. INTRODUCTION

1.1 Background

Rygate and West were engaged to prepare a drainage report to address the downstream major overland flow path for stage one of the proposed residential subdivision SF9366 at Narrawallee.

This report provides drainage calculations to determine the necessary drainage works within private property required to adequately control major flows from the proposed residential development and existing downstream residential catchment.

1.2 Catchment Description

The catchment contributing to the road sag point, at #108 Leo Drive, consists of the proposed residential development site and the existing downstream residential area shown in *Figure 1*.

The residential development site currently consists of moderately graded bushland that drains to the east. The development site grades in elevation from RL 31 at the south west corner to RL 8 in the eastern end of the site.

1.3 Proposed and Existing Development

The sag in Leo Drive is drained by a 750mm diameter pipe located within a 3m wide drainage easement passing through #108 Leo Drive and #4 Aries Place. Overland flows also pass through the private property generally within the existing easement.

The existing bushland upstream of the proposed development currently drains to Leo Drive.

The proposed piped drainage system for the new subdivision will be connected to the existing piped drainage system within Leo Drive. The allotment boundaries and road layout is shown on *Figure 2*.

1.4 Development Requirements

The existing and proposed storm water systems have been assessed using the minor / major system requirements as specified in Council's Development Control Plan DCP100.

The drainage system must cope with the minor storm event to avoid nuisance flooding. The major storm event must not be allowed to cause dangerous or unsafe conditions.

2. HYDROLOGICAL ANALYSIS AND FLOODING

The peak stormwater flows were estimated using the rational method contained in book eight of AR&R. Times of concentration were calculated using the kinematic wave equation. Hydrologic analysis was carried out to determine peak flows required to design the proposed overland flow channel. The model was used to estimate design flows under both pre-developed and developed conditions for the 100 year Average Recurrence Interval (ARI) events.

2.1 Pre-Development Peak Flow

Pre-Development catchment sub-areas and percentage impervious parameters are provided in *Table 2.1*. The calculated 100 year ARI runoff coefficient C_{100} , in accordance with 1.5.5(iii) of AR&R is 0.84. A 100 year ARI peak flow of 2.52 m³/s was determined using parameters in *Table 2.2*.

Pre-Development			
Catchment	Land Use	Area (ha)	Percentage Impervious
A	Existing Bushland	6.255	5%
B	Existing Residential	1.776	40%
	TOTAL	8.031	12.7%

Table 2.1

Pre-Development									
Land-Use	Flow Length (m)	Slope (%)	n*	Time (min)	Total Time (min)	Intensity (mm/hr)	Runnoff Coefficient	Area (ha)	100 yr ARI Peak Flow (m ³ /s)
Bushland	366	6	0.30	37.6	45.2	126.8	0.85	8.03	2.40
Residential	54	6	0.21	7.6					

Table 2.2 n* - surface roughness from Shoalhaven City Council DCP100.

2.2 Post-Development Peak Flow

Post-Development catchment sub-areas and percentage impervious parameters are provided in *Table 2.3*. The calculated 100 year ARI runoff coefficient C_{100} , in accordance with 1.5.5(iii) of AR&R is 0.92. The Post-Development time of concentration was calculated by adding the proposed residential development time of concentration and the estimated overland flow time through the existing residential area.

Calculations used to determine the 100yr ARI time of concentration within the proposed residential development are contained in Appendix A. The overland flow time through the existing residential area is estimated 1.2 minutes.

The 100 year ARI peak flow of $4.1 \text{ m}^3/\text{s}$ was determined using parameters in *Table 2.4*.

Post-Development			
Catchment	Land Use	Area (ha)	Percentage Impervious
A	Proposed Residential	6.819	40%
B	Existing Residential	1.776	40%
	TOTAL	8.595	40%

Table 2.3

Post-Development					
Land-Use	Total Time (min)	Intensity (mm/hr)	Runoff Coefficient	Area (ha)	100 yr ARI Peak Flow (m^3/s)
Residential	19	185.9	0.92	8.6	4.1

Table 2.4

3. HYDRAULIC ANALYSIS

Flows derived using the rational method, as detailed in **Section 2**, were used to estimate the hydraulic behaviour during the 100 year ARI storm event.

3.1 Existing Pipe Capacity

The capacity of the existing 750mm diameter pipe within the 3m wide drainage easement passing through #108 Leo Drive and #5 Aries Place has been calculated at 1.83 m³/s using Colebrook White equation and parameters in *Table 3.1*.

Pipe Length	73m	
U/S invert level	4.05	
D/S invert level	2.43	
Kinematic viscosity	1.14E-06	DCP 100 – 15 deg
Roughness (mm)	0.6	Concrete Pipes
Diameter (m)	0.75	
Sf (Hydraulic Gradient) (m/m)	0.02	
Velocity (m/s)	4.148	
Discharge (m ³ /s)	1.832	

Table 3.1

3.2 Overland Flow Path

It is proposed to construct a formalised overland flow path, as shown in **Figure 3**, to convey major flows up to the 100 year ARI event, from Leo Drive to Aries Place within the existing 3m wide drainage easement. An open channel flow capacity of 2.27 m³/s is required assuming the existing 750mm diameter pipe is just flowing full. A concrete lined open channel has been designed using the parameters in *Table 3.2*. Figure 3.1 shows a typical cross section of the proposed channel.

OPEN CHANNEL CALCULATIONS – MANNINGS EQUATION		
U/S invert level	6.38m	
D/S invert level	3.86m	
Length	67m	
Base Width	1.5m	
Side Slope	3:1	
Flow Depth	0.25	
Cross sectional area	0.562sq.m	
Wetted Perimeter	3.081m	
Longitudinal Slope	3.7%	
Mannings n	0.015	Concrete (trowel finish)
Hydraulic Radius	0.182	
Flowrate	2.32 m ³ /s	
Velocity	4.12m/s	
Velocity x Depth Product (m ² /m)	1.03	

Table 3.2

4. REFERENCES

The Institute of Engineers, Australia, Rainfall & Runoff, 2001

Shoalhaven City Council, Development Control Plan 100 - Engineering Design Specification

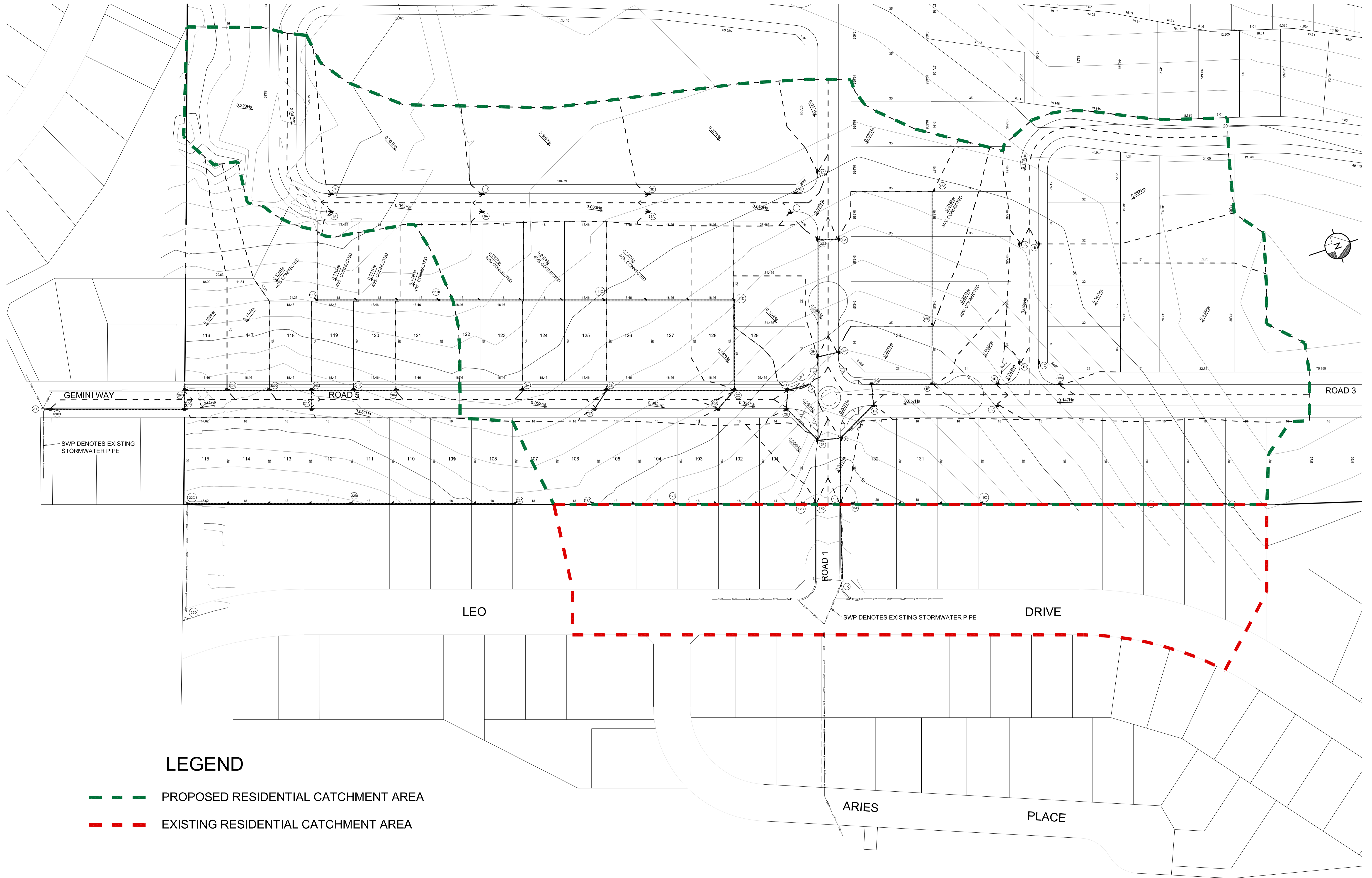
FIGURES



LEGEND

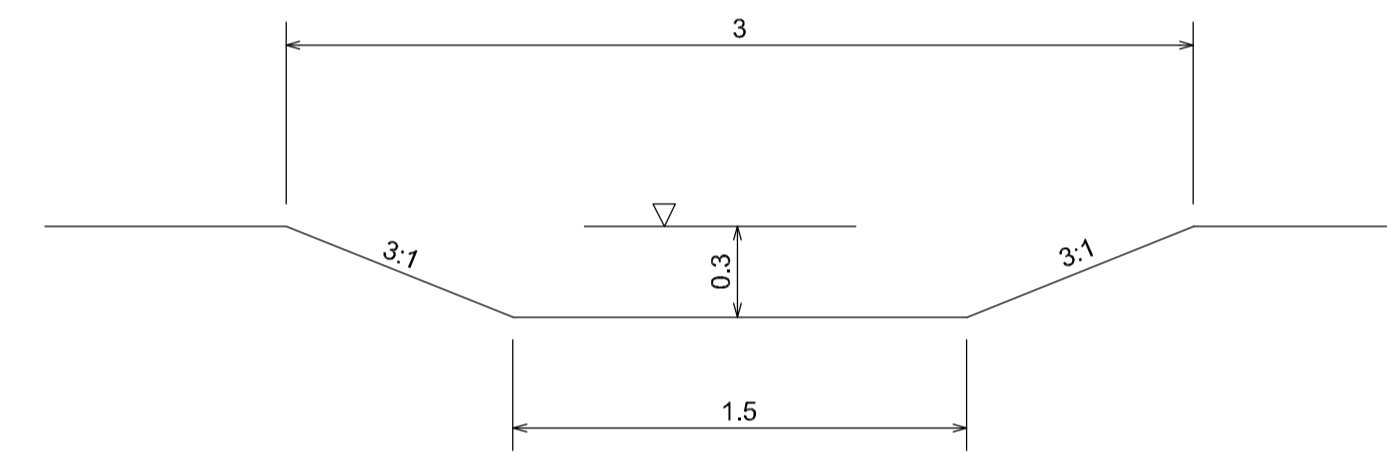
- - - EXISTING BUSHLAND CATCHMENT AREA 'A' -
- - - EXISTING RESIDENTIAL CATCHMENT AREA 'B'

FIGURE 2



LEGEND

- - - PROPOSED RESIDENTIAL CATCHMENT AREA
- - - EXISTING RESIDENTIAL CATCHMENT AREA



SECTION A - PROPOSED OPEN CHANNEL

Pit No.	Surface Type	Catchment Overland Flow Length	Catchment Overland Flow Slope	Catchment Overland Flow Roughness	Catchment Overland Flow Time	Gutter Length	Gutter Flow Time	Pit Tc (with Gutter)	Single Catchment Area	Catchment Pervious Area	Catchment Impervious Area	Pit C Factor	Pit Effective Area	Adopted Pit Tc	Rainfall Intensity	Total Catchment Flow	Catchment Flow Direct to Pipe	Catchment Flow to Pit Inlet	Pit Type	Specified Inflow to Pit	Bypass Received From Pit No.	Bypass Flow Received	Total Flow to Pit	Pit Inlet Inflow	Pit Bypass Flow	Bypass to Pit	Gutter Slope	Gutter Flow Width	Pit Gutter Flow Depth
10A					5			5		0.003	0.049	1	0.052	5	290.7	41.996		41.996	2.4m + gra		12A		52.894	37.092			1.403	0.065	
3E								6.56		0.233	0.144	0.188	0.071	6.561	266.1	52.478		52.478	2.4m + gra		10A	10.9			15.8	2E			
					5			5		0.004	0.067	1	0.071								3E	5.6			58.1	3F		Sag	40
		58.49	4.9	0.13	6.16	35.81	0.4	6.56		0.23	0.077	0	0													2.63			
1C								13.46		0.171	0.176	0.94	0.326	13.464	210.7	190.904		190.904	2.4m + gra		13A		290.904	190.904			Sag	40	
					5			5		0.003	0.064	1	0.067												100	1E			
		106.62	4.7	0.21	13.16	26.99	0.3	13.46		0.168	0.112	0.926	0.259								1C	100					7.34		
2A								8.96		0.138	0.111	0.932	0.232	8.961	241	155.366		155.366	2.4m + gra				155.366	88.683	66.7	2B		0	17.721
					5			5		0.001	0.02	1	0.021																
		77.92	5.4	0.17	8.71	22.57	0.25	8.96		0.137	0.091	0.926	0.211														1.14		
12A					5			5		0.003	0.049	1	0.052	5	290.7	41.996		41.996	2.4m + gra				41.996	31.098	10.9	10A		1.368	0.064
11C					5			5		0.005	0.096	1	0.101	5	290.7	81.569	83.689	-2.121	JP-600x600				-2.121	0					
3D								17.41		0.188	0.117	0.19	0.058	17.413	192.1	30.954		30.954	2.4m + gra		3C		46.251	33.438				0	20.077
					5			5		0.003	0.055	1	0.058									3D	15.3						
		89.78	3.1	0.3	17.4	1.09	0.01	17.41		0.185	0.062	0	0														2.76		
7A								5.42		0.002	0.035	0.998	0.037	5.417	283.1	29.036		29.036	2.4m + gra				29.036	23.422	5.6	3E		1.067	0.049
					5			5		0.002	0.034	1	0.036																
					5	37.51	0.42	5.42		0.001	0	0.926	0.001														2.92		
1B								12.56		0.178	0.209	0.944	0.366	12.555	215.8	219.106		219.106	2.4m + gra				219.106	108	100	1C		0	18.522
					5			5		0.005	0.093	1	0.098																
		85.55	3.5	0.21	12.55	0.8	0.01	12.56		0.173	0.116	0.926	0.268														3.77		
11B					5			5		0.004	0.072	1	0.076	5	290.7	61.378	62.974	-1.596	JP-600x600				-1.596	0					
3C								16.93		0.191	0.112	0.172	0.052	16.932	194.1	28.042		28.042	2.4m + gra		3B		51.771	36.474				0	22.213
					5			5		0.003	0.049	1	0.052									3C	23.7			15.3	3D		
		91.19	3.4	0.3	16.92	1.47	0.02	16.93		0.188	0.063	0	0														2.86		
8A					5			5		0.003	0.06	1	0.063	5	290.7	50.879		50.879	2.4m + gra		9A		69.808	45.904				1.785	0.077
					5			5		0.006	0.112	1	0.118	5	290.7	95.298		95.298	2.4m + gra		8A	18.9			23.9	3F			
1A					5			5		0.002	0.033	1	0.035	5	290.7	28.266	29.001	-0.735	JP-600x600				-0.735	0				1.898	0.08
11A					5			5		0.073	0.024	0.887	0.086	5	290.7	69.458		69.458	2.4m + gra				69.458	45.729	23.7	3C		0	24.118
					5			5		0.059	0.003	0.836	0.052																
					5			5		0.014	0.021	0.977	0.034																
9A					5			5		0.003	0.05	1	0.053	5	290.7	42.803		42.803	2.4m + gra		3A		59.841	40.913				1.681	0.074
					5			5		0.003	0.05	1	0.053	5	290.7	42.803		42.803	2.4m + gra		9A	17			18.9	8A			
3A								5.96		0.191	0.132	0.226	0.073	5.961	274.4	55.64		55.64	2.4m + gra				55.64	38.602	17	9A		1.629	0.072
					5			5		0.004	0.069	1	0.073																
		35.17	13	0.13	5	86.47	0.96	5.96		0.188	0.063	0	0														2.56		

Pipe Hydrology - Drainage 1 Return Period: 100yrs Location: Narrawallee

Pipe Connecting Pits\P(Downstream Upstream)	Pipe ID	Pipe Class	Pipe Diameter	Pipe Length	Pipe Mannings n	Pipe Flow Time	Pipe Design Tc	Pipe Rainfall Intensity	Upstream Pervious Areas	Upstream Impervious Areas	Upstream Single Catchment Areas	Total Upstream Areas	Total Effective Areas	Pipe Flow	Specified Inflow to Pipe	Total Pipe Flow
			(mm)	(m)		(min)	(min)	(mm/hr)	(Ha)	(Ha)	(Ha)	(Ha)	(Ha)	(l/s)	(l/s)	(l/s)
1K 1J	1J1K	Class 2 RRJ	600	33.7	0.013	0.06	19.06	185.67	2.499	3.818	0	6.348	5.099	2630		2630
1J 1I	1I1J	Class 2 RRJ	600	28.6	0.013	0.06	19	185.89	2.451	3.317	0	5.813	4.567	2358.3		2358.3
1J 17D	17D1J	Class 2 RRJ	225	10.9	0.013	0.04	5.77	277.26	0.008	0.158	0	0.229	0.226	174.3		174.3
1J 15D	15D1J	Class 2 RRJ	225	5	0.013	0.02	6.06	272.86	0.01	0.181	0	0.275	0.275	208.4		208.4
1I 1H	1H1I	Class 2 RRJ	600	20.5	0.013	0.09	14.16	207.06	0.718	1.213	0	1.988	1.904	1095.1		1095.1
1I 2F	2F1I	Class 2 RRJ	600	9.8	0.013	0.04	18.97	186.02	1.729	2.018	0	3.78	2.618	1352.9		1352.9
17D 17C	17C17D	PVC	225	5.1	0.009	0.03	5.75	277.68	0.004	0.078	0	0.166	0.166	128		128
15D 15C	15C15D	PVC	225	56.5	0.009	0.26	5.81	276.68	0.004	0.078	0	0.191	0.191	146.8		146.8
1H 1G	1G1H	Class 2 RRJ	600	8.8	0.013	0.04	14.12	207.26	0.601	1.079	0	1.931	1.847	1063.4		1063.4
2F 2E	2E2F	Class 2 RRJ	525	19.7	0.013	0.08	11.61	221.57	0.561	0.894	0	1.489	1.426	877.5		877.5
2F 3H	3H2F	Class 2 RRJ	375	36.6	0.013	0.11	18.86	186.44	1.135	1.027	0	2.258	1.16	600.5		600.5
17C 17B	17B17C	PVC	150	56.5	0.009	0.26	5.49	281.9	0.001	0.026	0	0.082	0.082	64.2		64.2
15C 15B	15B15C	PVC	150	72	0.009	0.33	5.48	282.01	0.001	0.026	0	0.082	0.082	64.2		64.2
1G 1F	1F1G	Class 2 RRJ	450	26.4	0.013	0.08	14.04	207.65	0.576	1.038	0	1.68	1.61	928.7		928.7
2E 2D	2D2E	Class 2 RRJ	525	7.9	0.013	0.03	11.57	221.79	0.484	0.824	0	1.455	1.392	857.4		857.4
3H 3G	3G3H	Class 2 RRJ	375	51.5	0.013	0.24	18.62	187.34	0.97	0.848	0	1.844	0.769	400.2		400.2
3H 4A	4A3H	Class 2 RRJ	375	9	0.013	0.09	10.83	226.72	0	0	0	0.318	0.298	187.7		187.7
17B 17A	17A17B	PVC	150	36	0.009	0.49	5	290.74	0	0	0	0.027	0.027	21.8		21.8
15B 15A	15A15B	PVC	150	35.5	0.009	0.48	5	290.74	0	0	0	0.027	0.027	21.8		21.8
1F 1E	1E1F	Class 2 RRJ	375	28.5	0.013	0.06	13.98	207.98	0.569	0.913	0	1.504	1.437	830.2		830.2
1F 16B	16B1F	PVC	150	26.6	0.009	0.09	5.76	277.44	0.001	0.027	0	0.11	0.11	84.8		84.8
2D 2C	2C2D	Class 2 RRJ	375	23.4	0.013	0.06	11.51	222.19	0.313	0.614	0	1.174	1.123	692.9		692.9
2D 5A	5A2D	Class 2 RRJ	375	9.6	0.013	0.18	6.56	266.11	0	0	0	0.134	0.131	97.2		97.2
3G 3F	3F3G	Class 2 RRJ	375	17	0.013	0.11	18.51	187.76	0.884	0.674	0	1.621	0.556	289.9		289.9
3G 6A	6A3G	Class 2 RRJ	375	8.8	0.013	0.14	10.58	228.5	0	0	0	0.197	0.187	118.8		118.8
1E 1D	1D1E	Class 2 RRJ	375	13.9	0.013	0.05	13.49	210.54	0.356	0.496	0	0.901	0.859	502.2		502.2
1E 14A	14A1E	Class 2 RRJ	375	8.8	0.013	0.14	5	290.74	0	0	0	0.147	0.147	118.7		118.7
1E 13A	13A1E	Class 2 RRJ	375	27.4	0.013	0.21	13.76	209.09	0	0	0	0.434	0.409	237.8		237.8
16B 16A	16A16B	PVC	150	58.4	0.009	0.76	5	290.74	0	0	0	0.028	0.028	22.6		22.6
2C 2B	2B2C	Class 2 RRJ	375	55.9	0.013	0.3	11.21	224.14	0.14	0.161	0	0.586	0.55	342.4		342.4
2C 11D	11D2C	PVC	225	39.6	0.009	0.12	6.35	268.9	0.011	0.201	0	0.289	0.289	215.9		215.9
2C 10A	10A2C	Class 2 RRJ	375	10.4	0.013	0.46	5	290.74	0	0	0	0.052	0.052	42		42
3F 3E	3E3F	Class 2 RRJ	375	8.1	0.013	0.06	18.45	187.98	0.651	0.53	0	1.558	0.493	257.4		257.4
1D 1C	1C1D	Class 2 RRJ	375	7.8	0.013	0.03	13.46	210.7	0.184	0.321	0	0.852	0.81	473.9		473.9
2B 2A	2A2B	Class 2 RRJ	375	36.9	0.013	0.44	8.96	241.04	0	0	0	0.249	0.232	155.4		155.4
2B 12A	12A2B	Class 2 RRJ	375	9.3	0.013	0.41	5	290.74	0	0	0	0.052	0.052	42		42
11D 11C	11C11D	PVC	225	56.9	0.009	0.24	6.11	272.18	0.006	0.105	0	0.212	0.212	160.3		160.3
3E 3D	3D3E	Class 2 RRJ	375	64.4	0.013	0.58	17.87	190.27	0.461	0.378	0	1.144	0.385	203.5		203.5

Pipe Connecting Pits\P(Downstream Upstream)	Pipe ID	Pipe Class	Pipe Diameter	Pipe Length	Pipe Mannings n	Pipe Flow Time	Pipe Design Tc	Pipe Rainfall Intensity	Upstream Pervious Areas	Upstream Impervious Areas	Upstream Single Catchment Areas	Total Upstream Areas	Total Effective Areas	Pipe Flow	Specified Inflow to Pipe	Total Pipe Flow
3E 7A	7A3E	Class 2 RRJ	375	13.8	0.013	0.87	5.42	283.08	0	0	0	0.037	0.037	29		29
1C 1B	1B1C	Class 2 RRJ	375	51.8	0.013	0.33	12.56	215.8	0.006	0.112	0	0.505	0.484	289.8		289.8
11C 11B	11B11C	PVC	225	72.5	0.009	0.55	5.56	280.71	0.002	0.033	0	0.111	0.111	86.6		86.6
3D 3C	3C3D	Class 2 RRJ	375	72.6	0.013	0.94	16.93	194.14	0.267	0.206	0	0.776	0.264	142.4		142.4
3D 8A	8A3D	Class 2 RRJ	375	7.8	0.013	0.28	5	290.74	0	0	0	0.063	0.063	50.9		50.9
1B 1A	1A1B	Class 2 RRJ	375	7.8	0.013	0.15	5	290.74	0	0	0	0.118	0.118	95.3		95.3
11B 11A	11A11B	PVC	150	53.5	0.009	0.56	5	290.74	0	0	0	0.035	0.035	28.3		28.3
3C 3B	3B3C	Class 2 RRJ	375	66	0.013	1.02	6.22	270.67	0.191	0.132	0	0.42	0.159	119.5		119.5
3C 9A	9A3C	Class 2 RRJ	375	7.8	0.013	0.34	5	290.74	0	0	0	0.053	0.053	42.8		42.8
3B 3A	3A3B	Class 2 RRJ	375	7.8	0.013	0.26	5.96	274.39	0	0	0	0.323	0.073	55.6		55.6